

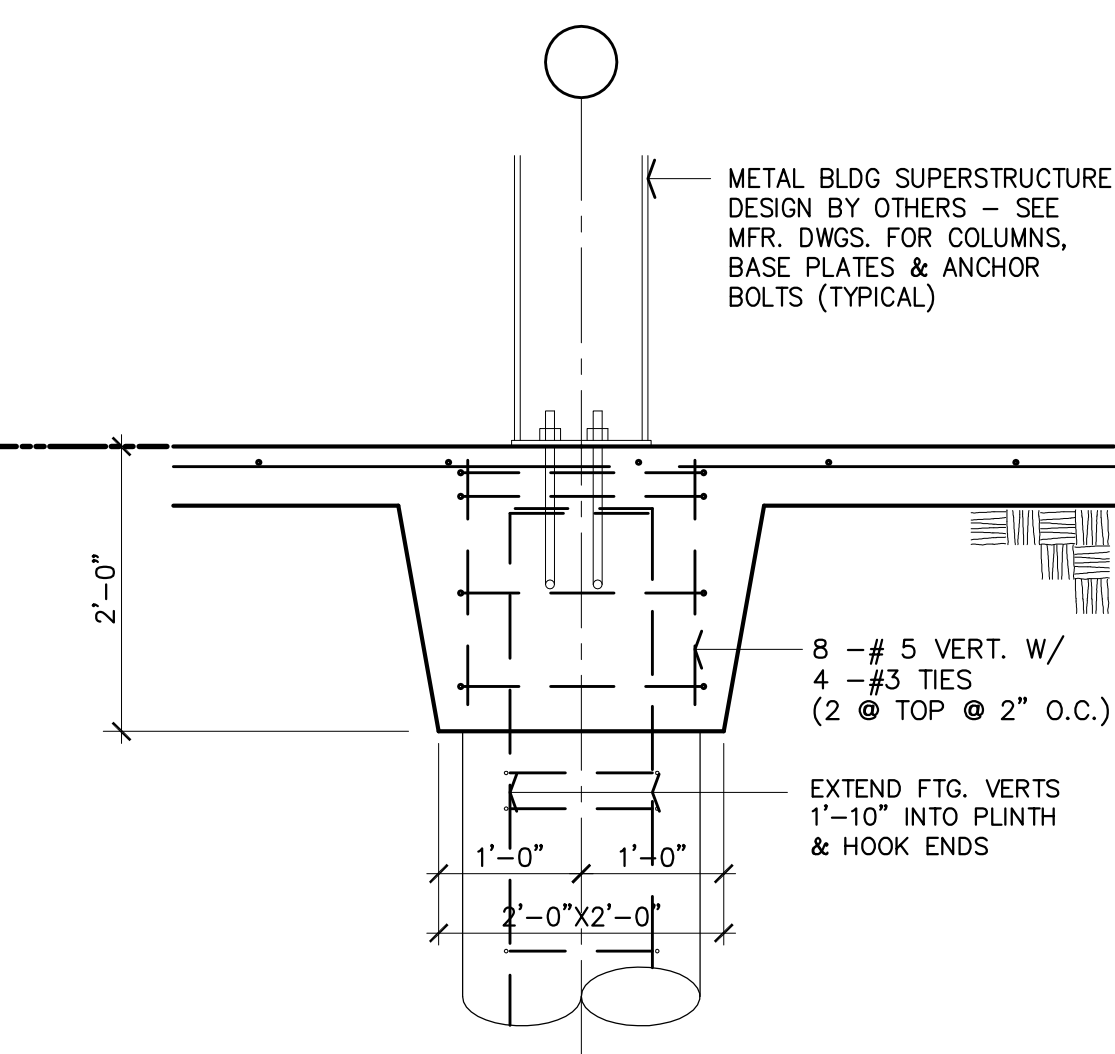
1A @ DOORS or WINDOW

DETAIL 3

DETAIL 2

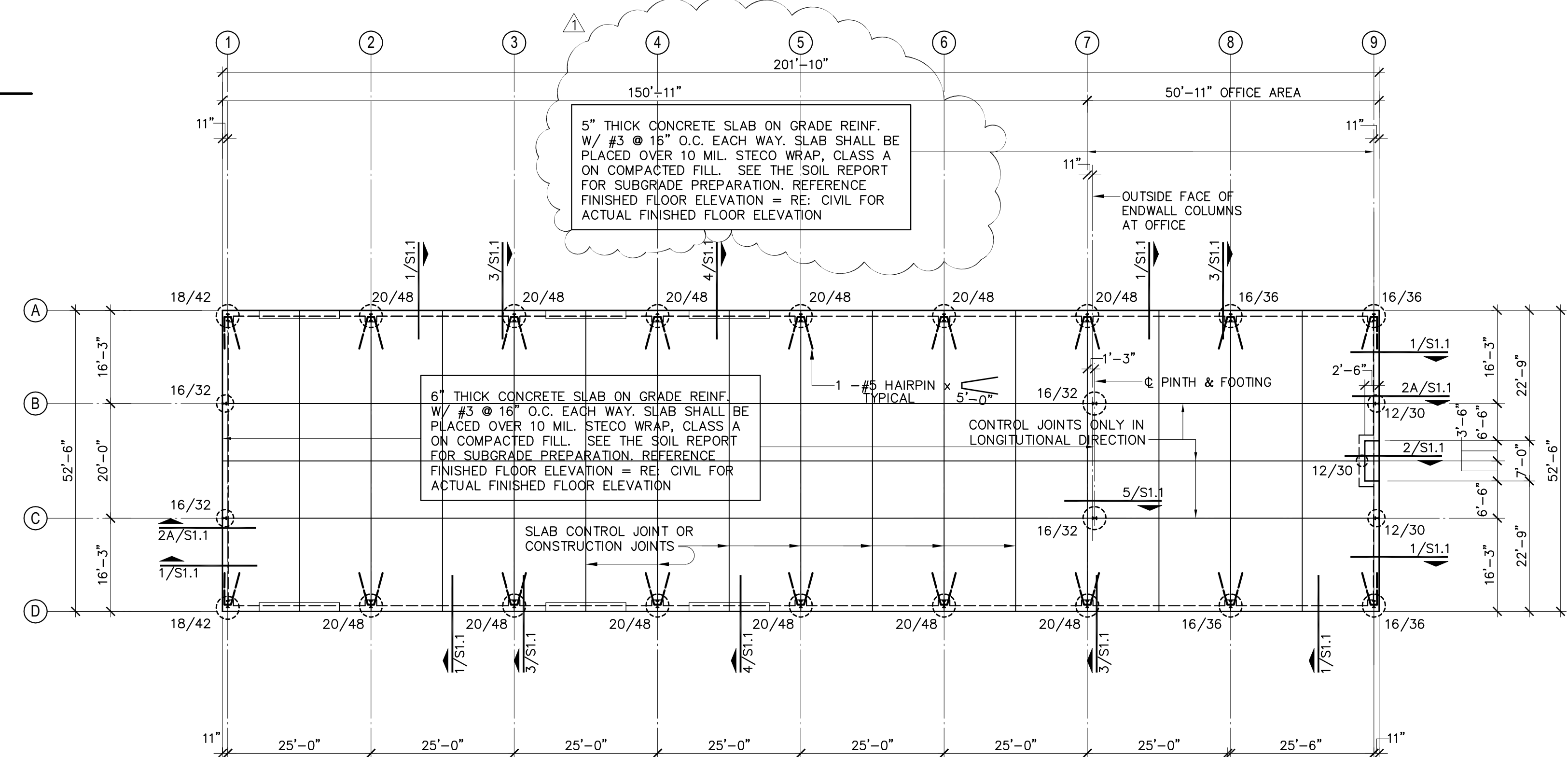
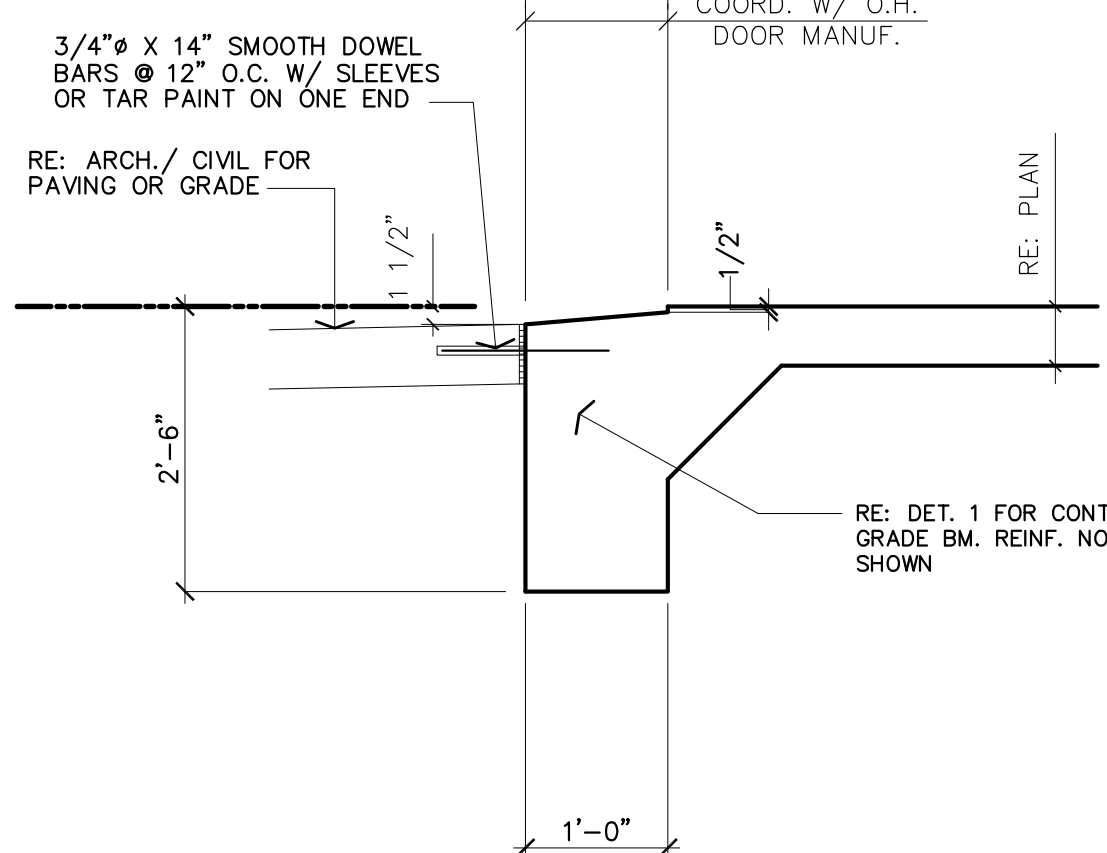
DETAIL 1

SCALE 3/4" = 1'-0" U.N.O.



DETAIL 5

DETAIL 4

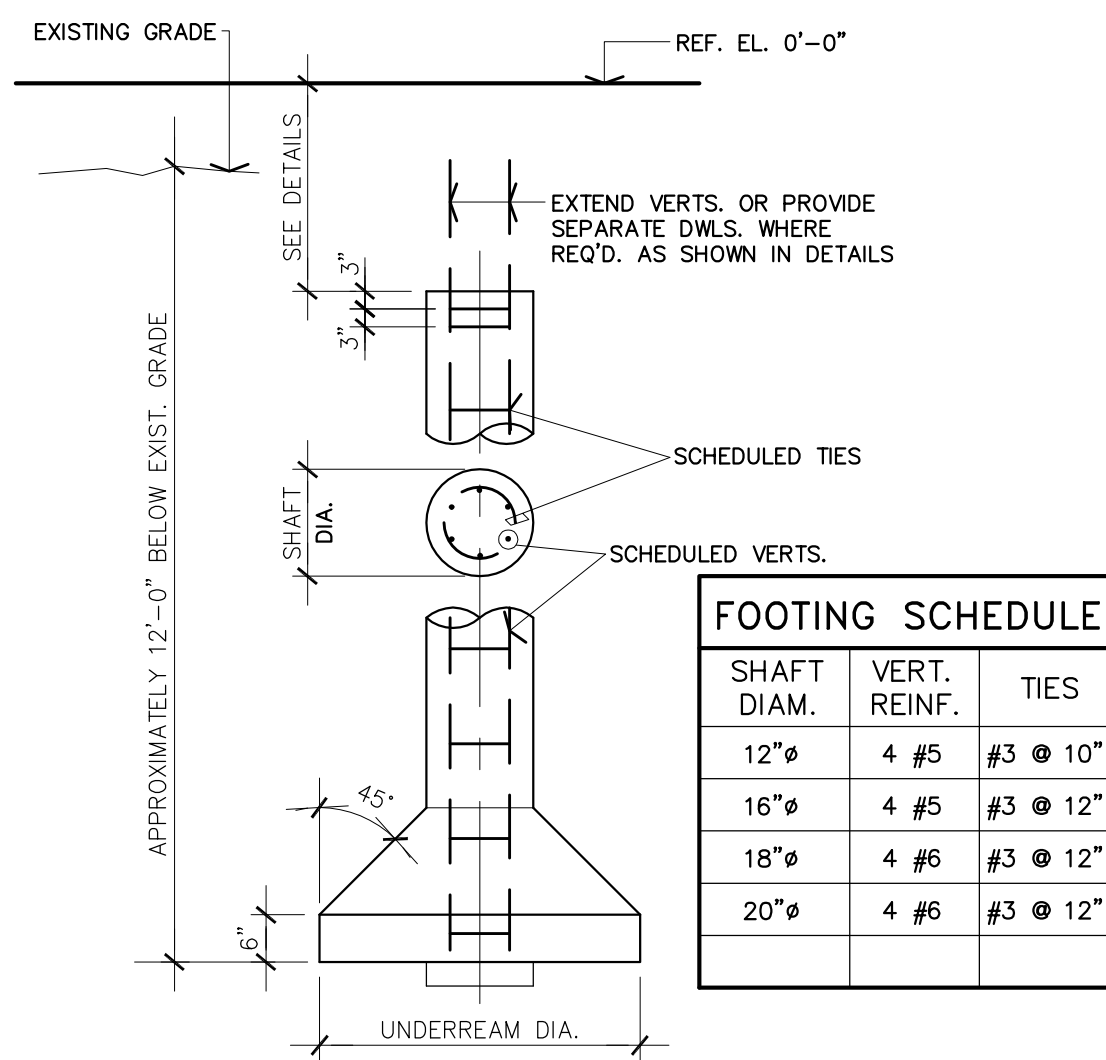


1 FOUNDATION PLAN

SCALE: 1/16" = 1'-0"

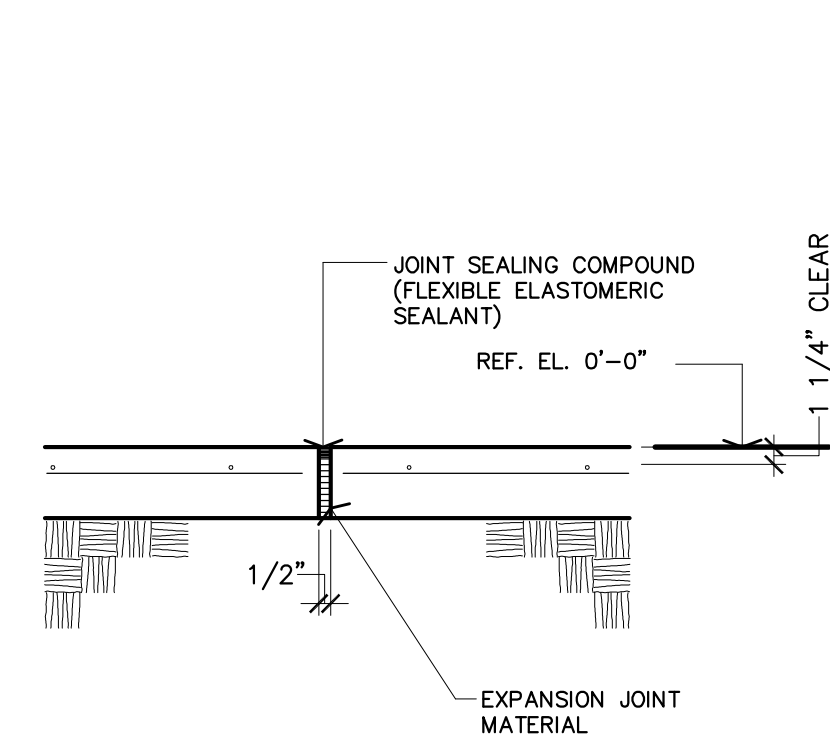
PLAN NOTES

1. TOP OF FOOTING (-2'-6") TYPICAL AT ALL EXTERIOR FTGS. U.N.O.
2. TOP OF FOOTING (-2'-0") TYPICAL AT ALL INTERIOR FTGS. U.N.O.



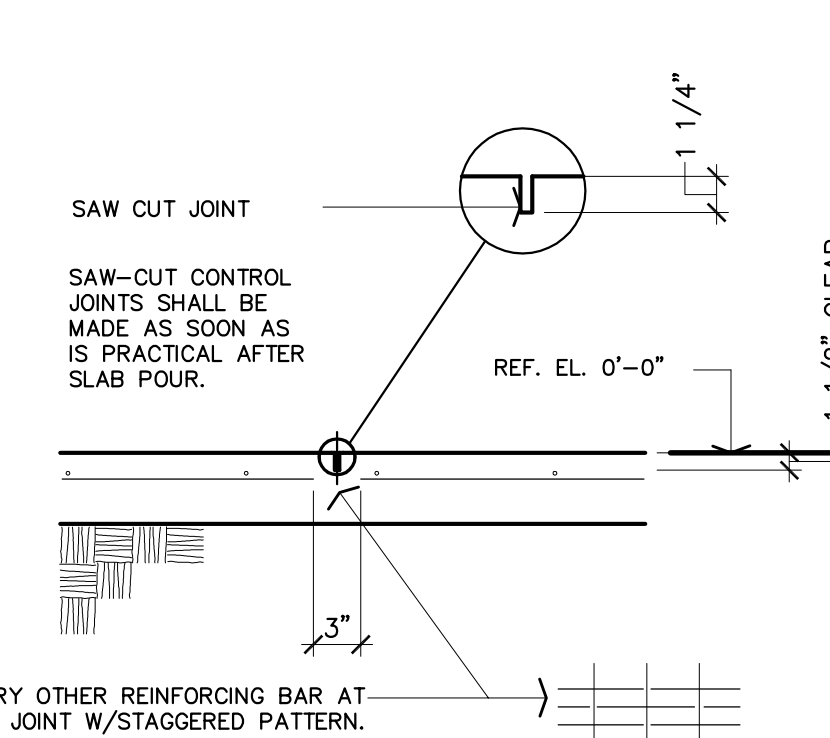
TYPICAL DRILLED FOOTING DETAIL

NO SCALE



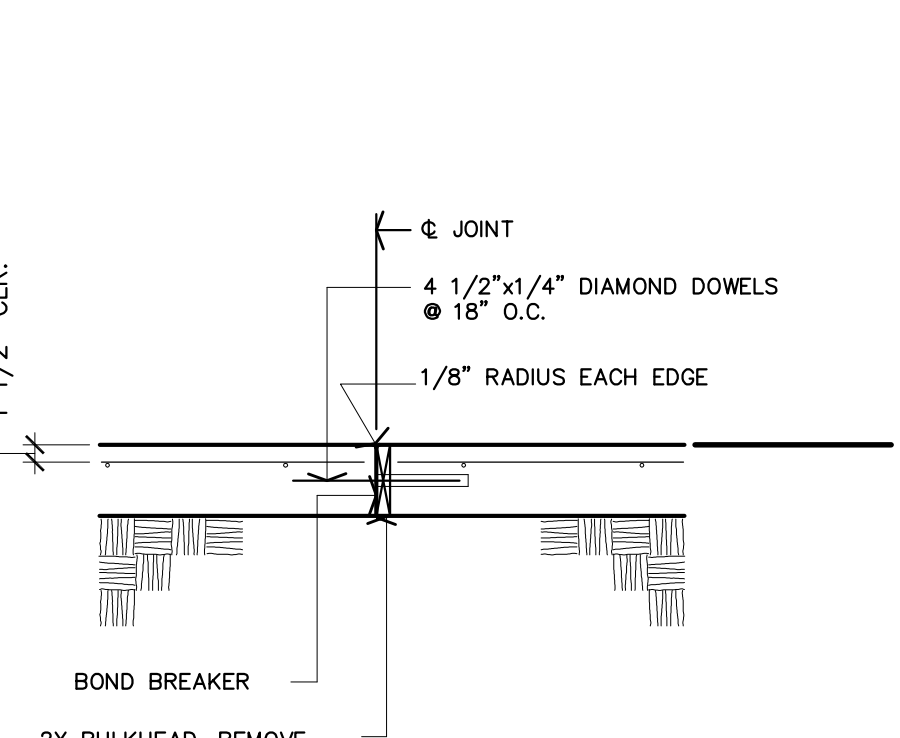
ISOLATION JOINT

NO SCALE



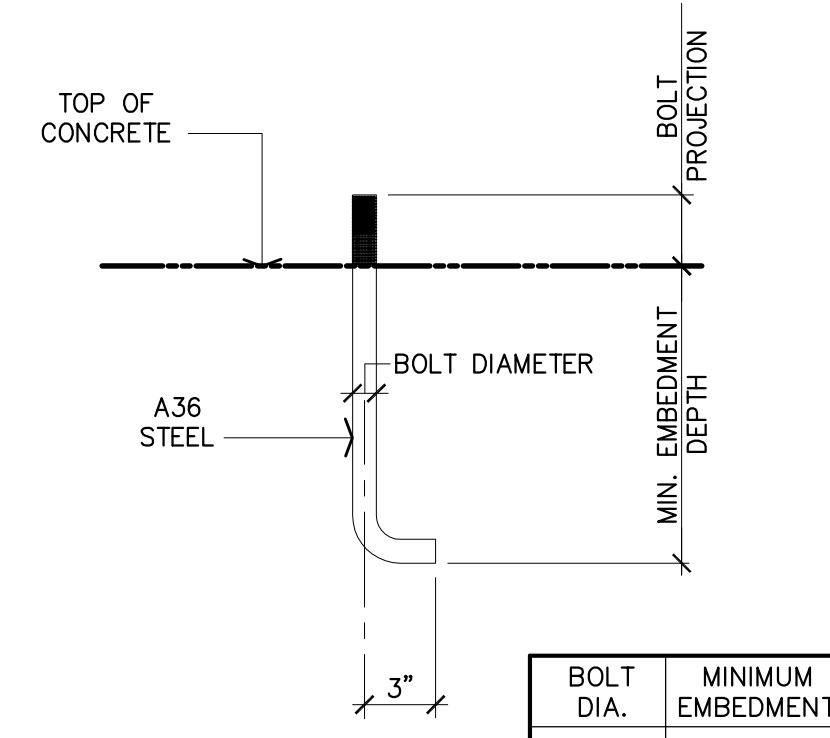
CONTROL JOINT

NO SCALE



CONSTRUCTION JOINT

NO SCALE



ANCHOR BOLT SCHEDULE

GENERAL NOTES

THE STRUCTURE IS DESIGNED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, 2006 EDITION. THE DESIGN LOADS ARE AS FOLLOWING.

ROOF LIVE LOAD	20 PSF
SNOW LOADS	PER SECTION 1608
WIND LOADS	WITH V = 110 MPH (3 SEC. GUST) EXPOSURE "C" AND IMPORTANCE FACTOR 1.0

THE FOUNDATIONS FOR THE STRUCTURE HAS BEEN DESIGNED BASED ON THE RECOMMENDATIONS OF THE GEOTECHNICAL ENGINEER, THE MURILLO COMPANY AS PRESENTED IN THEIR REPORT NUMBER GE039111, DATED NOVEMBER 2011 THE FOLLOWING ALLOWABLE BEARING PRESSURES WERE USED TO PROPORTION THE FOUNDATION AT A MINIMUM (12) TWELFVE FEET BELOW EXISTING GRADE.

DEAD LOAD	4,000 PSF
TOTAL LOAD	6,000 PSF

ANY FOUNDATION CONDITIONS WHICH DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT SHALL BE REPORTED IMMEDIATELY TO THE STRUCTURAL ENGINEER.

PRINCIPAL OPENINGS ARE SHOWN ON THE DRAWINGS. REFER TO ARCHITECTURAL MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR SLEEVES, CURBS, INSERTS, SMALL OPENINGS, ETC.

PROVIDE CRACK CONTROL JOINTS AT SLAB ON GRADE AREAS AS INDICATED ON FOUNDATION PLAN. REFER TO SHEET S2.1 FOR TYPICAL CONTROL AND CONSTRUCTION JOINT DETAILS.

CONCRETE IN THE FOLLOWING AREAS SHALL HAVE NATURAL SAND FINE AGGREGATES, NORMAL WEIGHT COARSE AGGREGATES CONFORMING TO ASTM C33, TYPE I PORTLAND CEMENT, AND SHALL HAVE THE FOLLOWING DESIGNATED COMPRESSIVE STRENGTH (F'c) IN 28 DAYS:

GRADE BEAMS AND SLAB ON GRADE	4000 PSI
FOOTINGS	3000 PSI

CORNER BARS SHALL BE PROVIDED AT ALL GRADE BEAM CORNERS AND INTERSECTIONS. BARS SHALL BE PLACED TOP AND BOTTOM AND SHALL BE 4 FOOT LONG BENT 90 DEGREES AT THE CENTER AND SIZED TO MATCH THE GRADE BEAM REINFORCING.

CONCRETE REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. NUMBER THREE BARS MAY CONFORM TO ASTM A615, GRADE 40.

REINFORCING BARS MAY NOT BE WELDED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE STANDARDS ACI 301-05. MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE IN ACCORDANCE WITH ACI 301-05.

CONCRETE COVER PROTECTION FOR REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ACI 318-05, SECTION 7.7.

HORIZONTAL JOINTS WILL NOT BE PERMITTED IN CONCRETE CONSTRUCTION, EXCEPT AS SHOWN ON THE STRUCTURAL DRAWINGS. ALL CONSTRUCTION JOINTS SHALL BE MADE WITH A VERTICAL BULKHEAD AND KEY WAY, AT LOCATIONS APPROVED BY THE STRUCTURAL ENGINEER.

ANCHOR BOLTS SHALL CONFORM TO ASTM A307, UNLESS NOTED OTHERWISE.

THE METAL BUILDING MANUFACTURER/DESIGNER SHALL DESIGN ALL RIGID FRAME COLUMN BASES AS "PINNED". THE FOUNDATION HAS NOT BEEN DESIGNED FOR ANY FRAME INDUCED MOMENTS.

THE METAL BUILDING MANUFACTURER/DESIGNER SHALL SUBMIT ALL SUPERIMPOSED LOADS ON THE FOUNDATION TO THE STRUCTURAL ENGINEER FOR APPROVAL AND COORDINATION PRIOR TO ANY FABRICATION.

THE FOUNDATION FOR THE METAL BUILDING SUPERSTRUCTURE WERE DESIGNED BASED ON ASSUMED REACTIONS. THE STRUCTURAL ENG. MUST COORDINATE FOOTING SIZES W/ ACTUAL METAL BUILDING REACTIONS COMPUTED BY THE METAL BUILDING MANUFACTURER THEREFORE, THE CONTRACTOR SHALL SUBMIT BUILDING REACTIONS FOR REVIEW AND APPROVAL PRIOR TO SUBMITTING THE FOUNDATION REINFORCEMENT SHOP DRAWING. UNDER NO CIRCUMSTANCES SHALL THE CONTRACTOR START FOUNDATION CONSTRUCTION PRIOR TO METAL BUILDING REACTION COORDINATION.